LOCATION HYDRAULIC STUDY AND SUMMARY FLOODPLAIN ENROACHMENT REPORT

FOR THE

RAILROAD AVENUE BRIDGES PROJECT

Over Fornat Wash (Br. No. 56C0099) (Federal Aid Project No. BRLO-5956(228)

Over East Channel Stubbe Wash (Br. No. 56C0101) (Federal Aid Project No. BRLO-5956(229)

Submitted to:

CALIFORNIA DEPARTMENT OF TRANSPORTATION, DISTRICT 8

464 W. 4th Street San Bernardino, CA 92401

Prepared for:

COUNTY OF RIVERSIDE DEPARTMENT OF TRANSPORTATION

3525 14th Street Riverside, CA 92501

Prepared by:

AGUILAR CONSULTING, INC.

2155 Chicago Avenue, Suite 304 Riverside, CA 92507 (951) 300-1431

LOCATION HYDRAULIC STUDY AND SUMMARY FLOODPLAIN ENROACHMENT REPORT FOR THE RAILROAD AVENUE BRIDGES PROJECT RIVERSIDE COUNTY, CALIFORNIA

This report has been prepared by or under the direction of the following registered civil engineer who attests to the technical information contained herein. The registered civil engineer has also judged the qualifications of any employees that have provided data and calculations upon which the recommendations, conclusions, and decisions are based.



Ceazar V. Aguilar, PE 41679

Memorandum

Making Conservation a California Way of Life.

AARON P. BURTON To:

SENIO ENVIRONMENTAL PLANNER

LOCAL ASSISTANCE - ENVIRONMENTAL SUPPORT

March 27, 2020 Date:

FPN: 5956(225-228 & 239) File:

LHS-SFER Report for

Railroad Ave Bridge Project 08-Riv-10 near PM 107-110 Fornat Wash - Stubbe Wash

From: ALAN BISI Office Chief

Hydraulics, MS 1065

Subject: LOCATION HYDRAULIC STUDY REPORT WITH SUMMARY FLOODPLAIN ENCROACHMENT **REPORT**

District 8 Hydraulics Unit has reviewed the revised Location Hydraulic Study and Summary Floodplain Encroachment Report for above mentioned project, prepared by Aguilar Consulting, Inc., dated February 2020. The document appears to address all hydraulic information adequately. We concur with the response provided and recommend acceptance of the report as an IGR supporting document.

If you have any questions or would like to discuss any of the above items further, please feel free to contact the Project Engineer, Michael Huynh, at (909) 806-2524 or myself, at (909) 383-4624

LOCATION HYDRAULIC STUDY AND SUMMARY FLOODPLAIN ENROACHMENT REPORT FOR THE RAILROAD AVENUE BRIDGES PROJECT RIVERSIDE COUNTY, CALIFORNIA

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LOCATION HYDRAULIC STUDY AND SUMMARY FLOODPLAIN ENROACHMENT REPORT FOR THE RAILROAD AVENUE BRIDGES PROJECT RIVERSIDE COUNTY, CALIFORNIA

I. INTRODUCTION

Aguilar Consulting Inc. has prepared the Location Hydraulic Study (LHS) and Summary Floodplain Encroachment Report (SFER) in support of the proposed Railroad Avenue Bridges Project.

The County of Riverside (County), in cooperation with California Department of Transportation (Caltrans), proposes to replace the following two (2) existing scour critical and structurally deficient timber bridges along Railroad Avenue near Whitewater in Riverside County, California (see Exhibit "A" and Exhibit "B"):

- Railroad Avenue Bridge over Fornat Wash (Br. No. 56C0099) (Federal Aid Project No. BRLO-5956(228)
- Railroad Avenue Bridge over East Channel Stubbe Wash (Br. No. 56C0101) (Federal Aid Project No. BRLO-5956(229)

II. PROJECT DESCRIPTION

Railroad Avenue is an approximately 5-mile stretch of road that runs parallel to Interstate 10 (I-10) and the Union Pacific Railroad (UPRR). It connects the Haugen-Lehmann Way and I-10 at the east end and Main Street and I-10 at the west end. It mostly serves the sparsely populated Cabazon community. The average daily traffic (ADT) volume is approximately 339 vehicles. Periodically, the road carries detoured traffic from the heavily traveled I-10 when the freeway is temporarily closed for construction or emergency incidents. The road also serves as an access route for UPRR and utility maintenance crews. Therefore, it is important to maintain this frontage road in sound condition at all times.

The existing timber bridges carry two lanes (one lane in each direction) of traffic over Fornat and East Channel Stubbe Washes. The timber bridges are approximately 59 feet long and are 32 feet wide from curb-to-curb. The County proposes replacing the existing two 2-lane timber bridges along Railroad Avenue with new 2-lane modern bridges with a curb-to-curb roadway width of 32 feet at the same locations.

The bridges are listed in the federal Eligible Bridge List (EBL), as "Structurally Deficient (SD)" with a low Sufficiency Rating (SR) between 59.1 and 62.9. A sufficiency rating is essentially an overall rating of a bridge's fitness for the duty that it performs. The rating is based on a bridge's structural evaluation, functional/geometric obsolescence, and its essentiality to the public. A low sufficiency rating may be due to structural defects, narrow lanes, low vertical clearance, or any of many possible issues. A bridge is healthy when its SR is more than 80.0. Bridges with SR equal to or less than 80.0 and more than 50.0 require rehabilitation or widening. When the SR falls less than 50.0, bridge replacement shall be considered for public safety. Although the Railroad Avenue bridges carry a status flag of SD with SR ratings between 50 and 80 (qualifying for major rehabilitation), it was determined that the bridges are well beyond their 50-year service life and it would be more cost-efficient to replace the bridges. Additionally, a scour Plan of Action (POA) was

LOCATION HYDRAULIC STUDY AND SUMMARY FLOODPLAIN ENROACHMENT REPORT FOR THE RAILROAD AVENUE BRIDGES PROJECT RIVERSIDE COUNTY, CALIFORNIA

performed on the bridges by the County in 2013. The POA recommended total replacement of the bridges as the most cost-effective option due to the extent of the scour, structural instability and deterioration of various timber bridge elements.

The proposed project would replace the existing 2-lane timber bridges with new 2-lane modern bridges. The proposed road width would consist of two 12-foot-wide travel lanes, one lane in each direction, and a 4-foot-wide shoulder on each side. Modern traffic barriers/railings meeting current CALTRANS safety design standards would be constructed. The proposed bridges would be approximately 60 feet long depending on the channel hydraulic capacity and water surface freeboard requirements. Potentially the elevation of Fornat Wash Bridge may increase, but by no more than two feet to meet freeboard requirements. The East Channel Stubbe Wash Bridge elevation would remain the same. Additionally, approach roadway improvements would be provided and channel improvements would be administered to avoid future scour problems. It is envisioned that the channel bottom will remain earthen.

Existing underground utilities along the north side of Railroad Avenue and suspended utilities (a 4-inch gas line and a telephone line) along the north side of the East Channel Stubbe Wash bridge would be affected by construction and may require relocation.

All construction activities would be conducted within the existing roadway right of way with construction staging and material laydown areas on the roadway itself. Railroad Avenue between the two bridges to be replaced would be closed for continuous traffic during construction. The construction duration will be further determined during the project development. It is envisioned that the two bridges will be constructed one at a time to allow access to UPRR facilities and adjacent utilities from the Haugen-Lehmann Way/I-10 Interchange or the Main Street/I-10 Interchange. A Traffic Management Plan (TMP) would be prepared to address closure of the road and access to local utilities and properties.

The proposed construction would require a temporary construction easement (TCE) from UPRR for access to the channel bottom. However, construction activities are expected to stay at least 50 feet from the live rail tracks to eliminate any effects on railroad operations. The Railroad Avenue bridges abut adjacent State Bridges (Br. No. 56-166 and Br. No. 56-168) that carry I-10 traffic over the same washes. Structural modifications to the State Bridges are not anticipated; however, this will be evaluated during design. An encroachment permit from Caltrans District 8 would be obtained prior to construction.

III. FLOODPLAIN ENCROACHMENT

The FEMA definition of **encroachment** states that "construction, placement of fill, or similar alteration of topography in the floodplain that reduces the area available to convey floodways." The Federal Highway Administration (FHWA) defines it as "an action within the limits of the base floodplain". Review of the Federal Emergency Management Agency (FEMA) Flood Insurance Rate Map (FIRM) Number 06065C0865G effective August 28, 2008 (Exhibit "D"), the following has been found:

LOCATION HYDRAULIC STUDY AND SUMMARY FLOODPLAIN ENROACHMENT REPORT FOR THE RAILROAD AVENUE BRIDGES PROJECT RIVERSIDE COUNTY, CALIFORNIA

- It is indicated that the Fornat Wash Bridge is within a designated Zone "X" (unshaded), which means the area is outside the 0.2% annual chance floodplain. Therefore, the Fornat Wash Bridge <u>does not encroach</u> into any defined floodplains or floodways as shown on Exhibit "D".
- It is indicated that the East Channel Stubbe Wash Bridge is within a designated Special Flood Hazard Zone "A" known as the Stubbe Canyon Wash, which means that the area is subject to the 1% annual chance floodplain, with no known base flood elevations. Therefore, the East Stubbe Wash Bridge **encroaches** into the 1% annual chance flood hazard zone as shown on Exhibit "D", and thus subject to inundation by the 100-year flood event. Construction of the bridge **will not** have any impact to the existing floodplain.

Caltrans has prepared a hydrology study for Fornat Wash and East Channel Stubbe Wash showing the following flow rates for the 100-year and 50-year flood events (Ref. 2):

- Fornat Wash
 - \circ Q₁₀₀ = 2,500 CFS
 - \circ Q₅₀ = 1,800 CFS
- East Channel Stubbe Wash
 - \circ Q₁₀₀ = 4,200 CFS
 - $Q_{50} = 3,100 \text{ CFS}$

The flow rates were determined using the "Troxell method", an outdated hydrologic calculation method. A floodplain analysis using the HEC-RAS model was performed for Fornat Wash and East Channel Stubbe Wash using Caltrans' flow rates based on the proposed bridge footprint at Railroad Avenue. The results indicate that the 100-year and 50-year flow rates are contained within the cross section of the channel and the proposed bridge section with the following bridge freeboard:

- Fornat Wash
 - \circ For Q₁₀₀ = 2,500 CFS, the average freeboard at the upstream side of the bridge is approximately 0.13 feet with a maximum freeboard of 0.26 feet at the northwest bridge abutment while at the downstream side of the bridge, it is approximately 0.72 feet with a maximum freeboard of 1.28 feet at the southwest bridge abutment.
 - For $Q_{50} = 1,800$ CFS, the average freeboard at the upstream side of the bridge is approximately 0.82 feet with a maximum freeboard of 1.31 feet at the northwest bridge abutment while at the downstream side of the bridge, it is

LOCATION HYDRAULIC STUDY AND SUMMARY FLOODPLAIN ENROACHMENT REPORT FOR THE RAILROAD AVENUE BRIDGES PROJECT RIVERSIDE COUNTY, CALIFORNIA

approximately 1.60 feet with a maximum freeboard of 2.16 feet at the southwest bridge abutment.

• East Channel Stubbe Wash

- o For $Q_{100} = 4,200$ CFS, the average freeboard at the upstream side of the bridge is approximately 5.54 feet with a maximum freeboard of 5.93 feet at the northeast bridge abutment while at the downstream side of the bridge, it is approximately 7.74 feet with a maximum freeboard of 8.18 feet at the southeast bridge abutment.
- o For Q₅₀ = 3,100 CFS, the average freeboard at the upstream side of the bridge is approximately 6.91 feet with a maximum freeboard of 7.33 feet at the northeast bridge abutment while at the downstream side of the bridge, it is approximately 8.91 feet with a maximum freeboard of 9.34 feet at the southeast bridge abutment.

The overtopping flow rates at the two bridges were determined to be approximately 3,550 CFS for Fornat Wash and 12,800 CFS for East Channel Stubbe Wash using a flood frequency curve (FFC) as shown on Exhibit "E.1" and "E.2". These flows are equivalent to approximately 188-year and 840-year flood events, respectively.

IV. REFERENCES

- 1. Federal Emergency Management Agency (FEMA) Map Services.
- 2. Caltrans; "Drainage Report for the Design Project on State Highway Route 26 in Riverside County from East City Limits of Cabazon to West Junction of Route 187", VIII-Riv-26-C, 8V10H446.1, April 27, 1962.

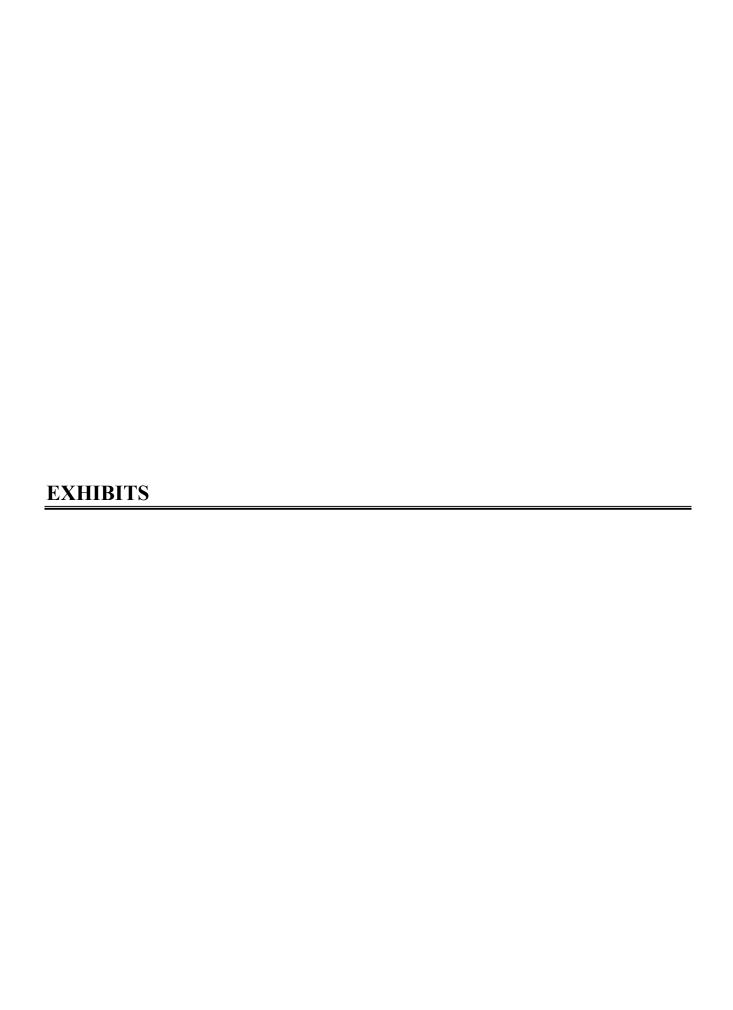




EXHIBIT "A" - REGIONAL LOCATION MAP

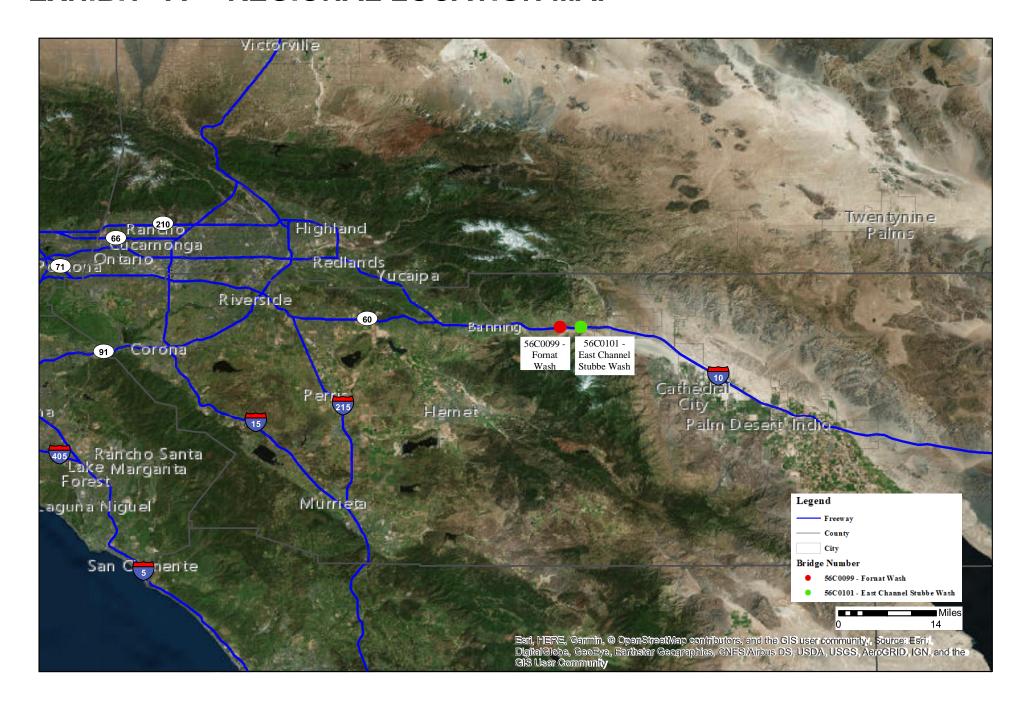




EXHIBIT "B" - PROJECT LOCATION MAP

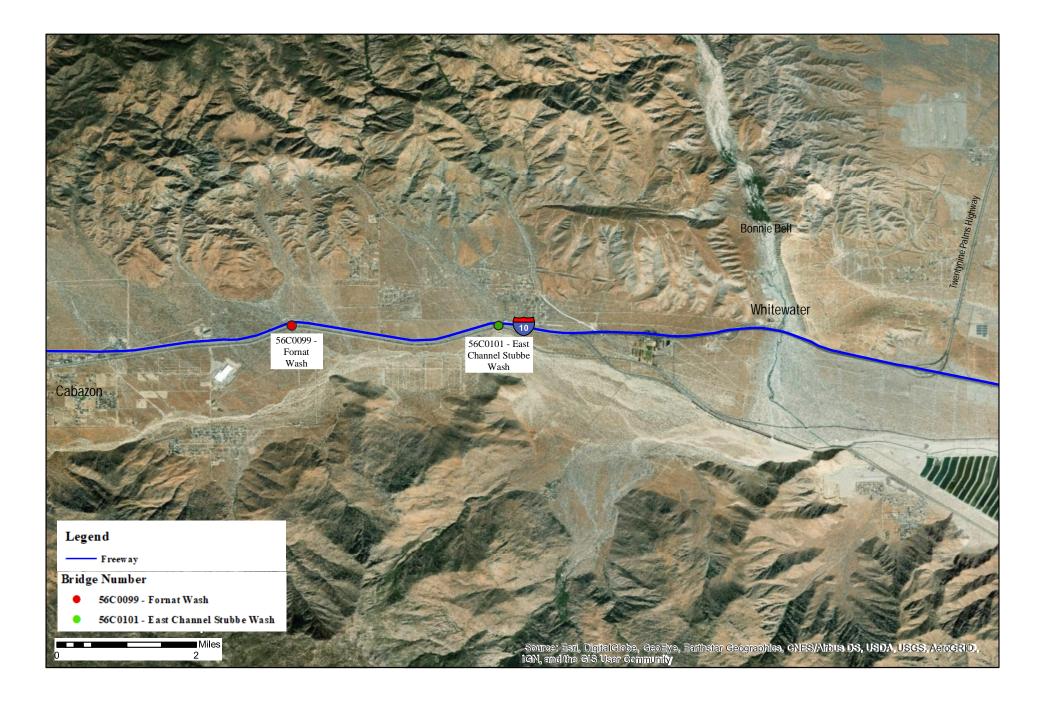
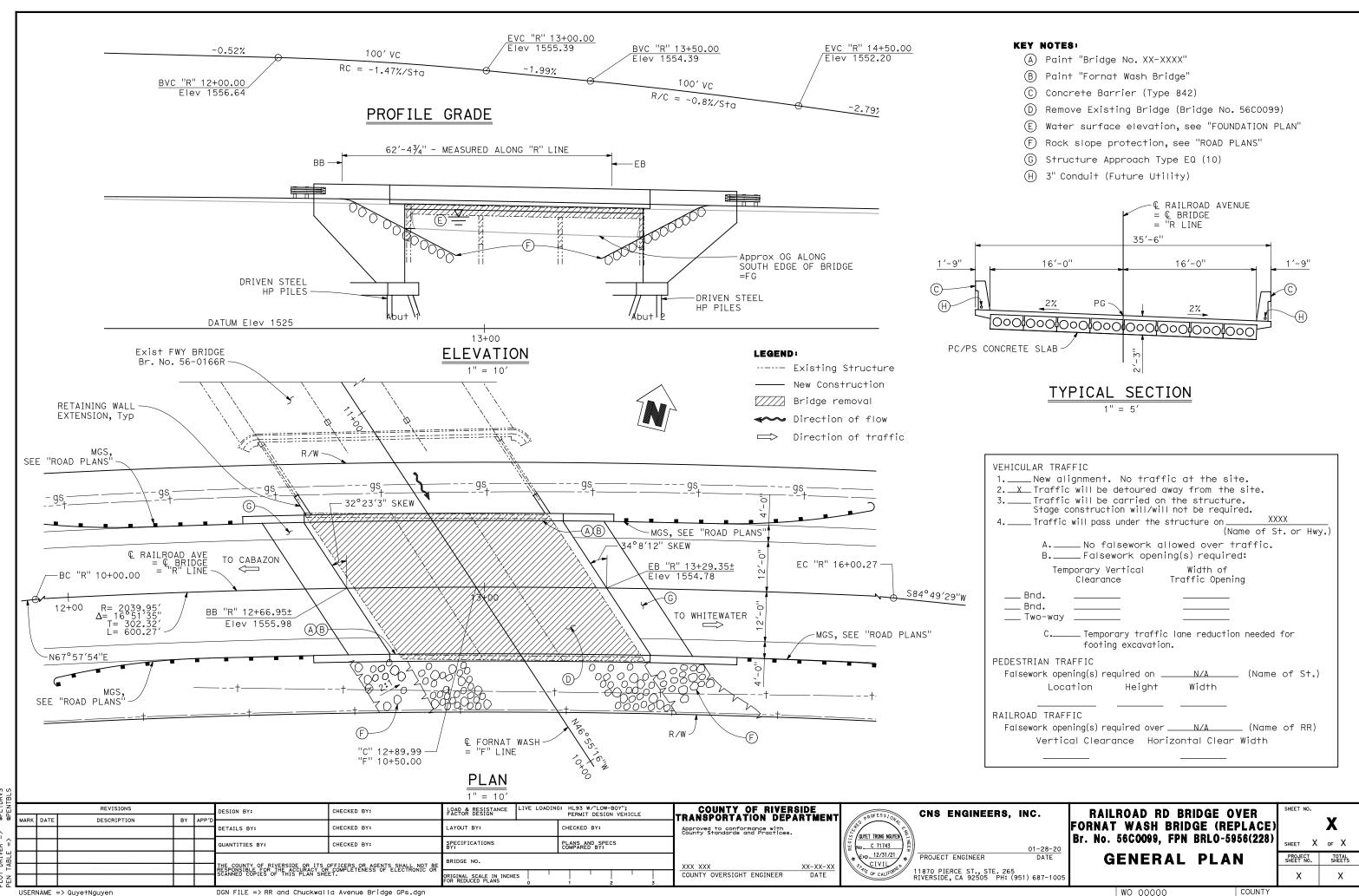
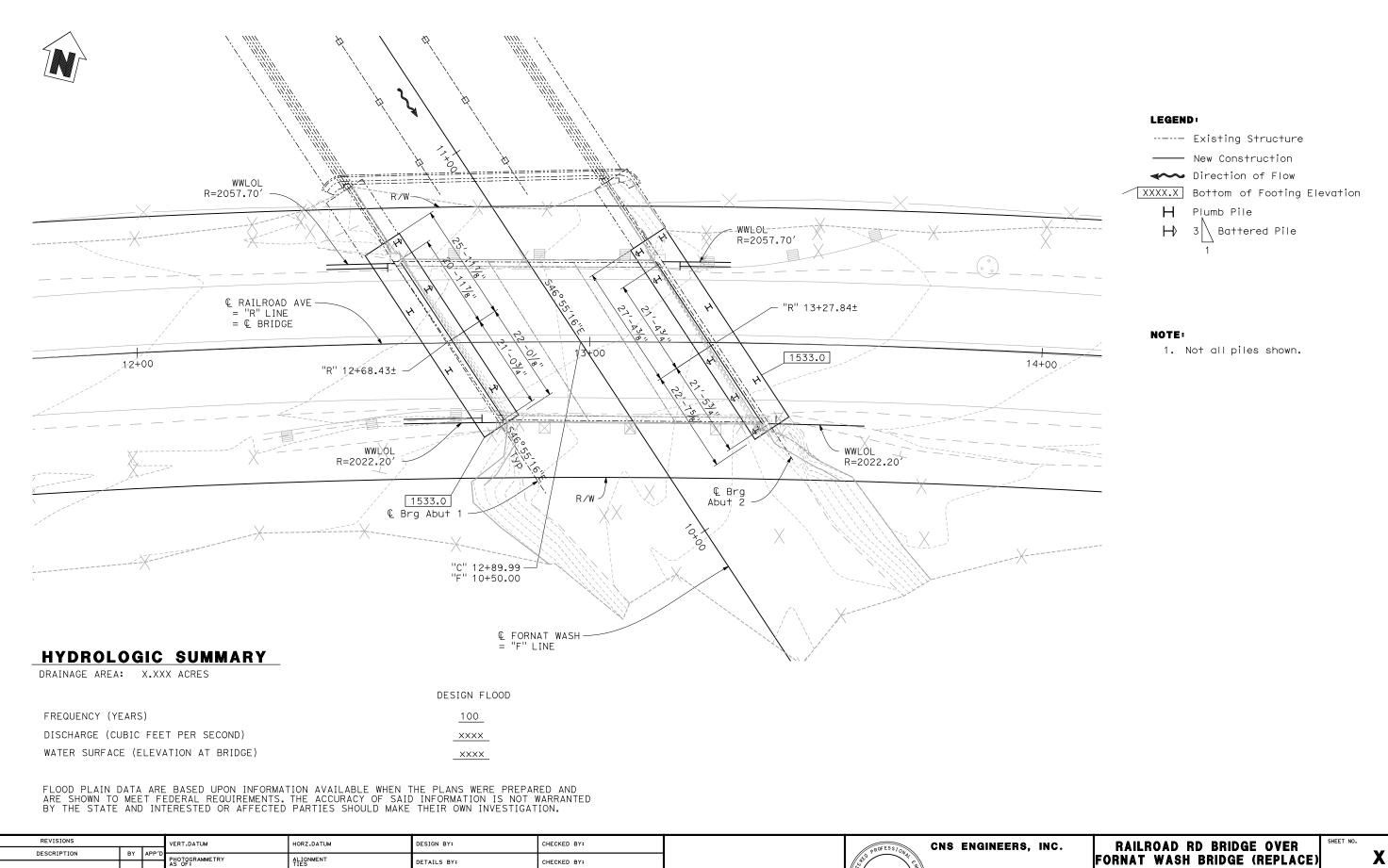


EXHIBIT "C": BRIDGE GENERAL PLANS AND BRIDGE GEOMETRIC APPROVAL DRAWINGS FOR THE PROPOSED RAILROAD AVENUE BRIDGES



WO 00000



PLOT DRIVER => \$PLTDRVS PEN TABLE => \$PENTBLS

USERNAME => Quye+Nguyen

THE COUNTY OF RIVERSIDE OR ITS OFFICERS OR AGENTS SHALL NOT BE RESPONSIBLE FOR THE ACCURACY OR COMPLETENESS OF ELECTRONIC OR SCANNED COPIES OF THIS PLAN SHEET. DGN FILE => RR and Chuckwalla Avenue Bridge FPs.dgn

DRAFTED BY:

CHECKED BY:

QUANTITIES BY:

RIGINAL SCALE IN INCHES

CHECKED BY:

SURVEYED BY:

FIELD CHECKED BY:

WO 00000

QUYET TRONG NGUYEN

PROJECT ENGINEER

11870 PIERCE ST., STE. 265 RIVERSIDE, CA 92505 PH: (951) 687-1009

No. C 71743

COUNTY FILE No.

SHEET X OF X

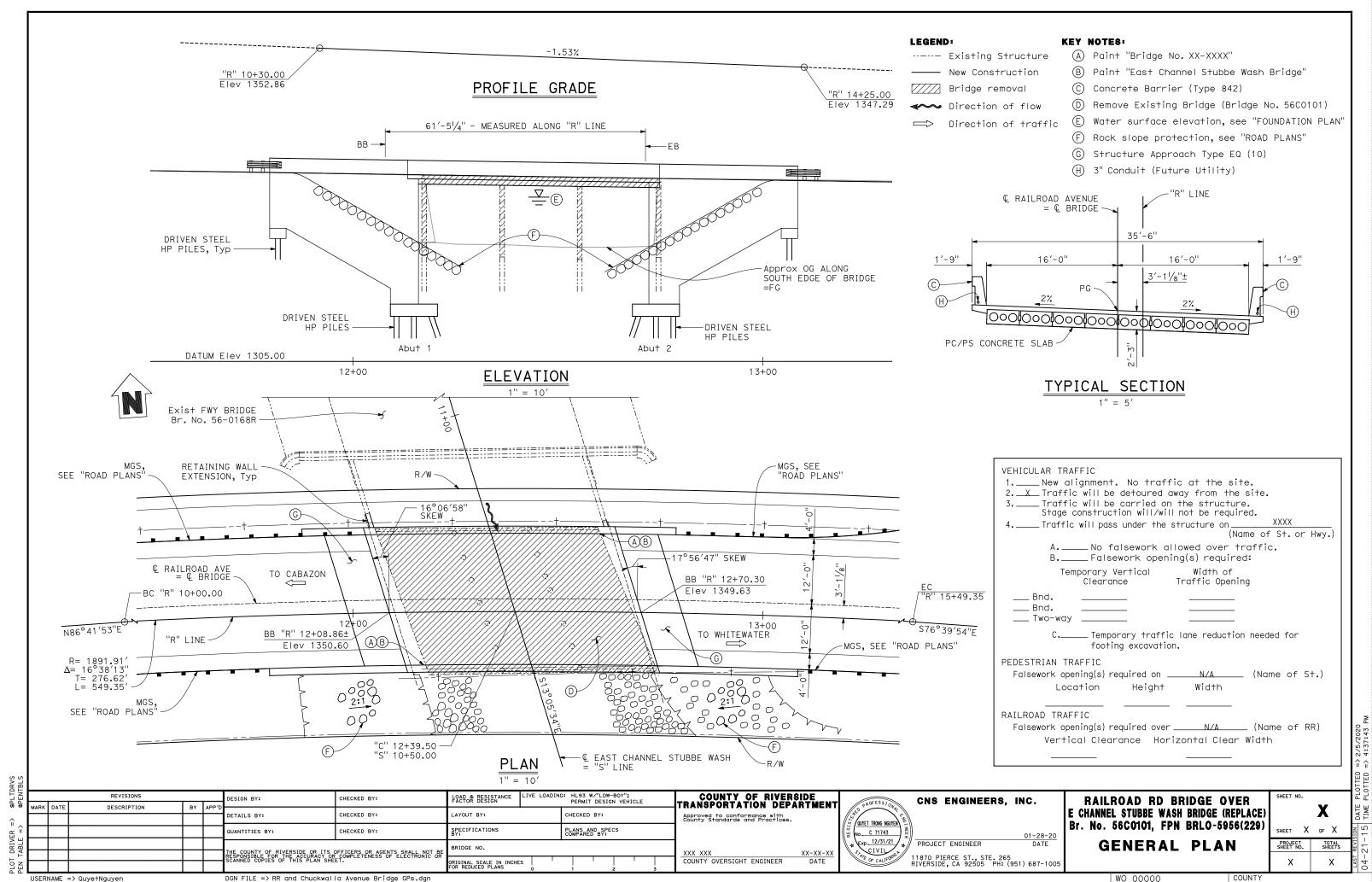
PROJECT TOTAL
SHEET NO. SHEETS

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Br. No. 56C0099, FPN BRLO-5956(228)

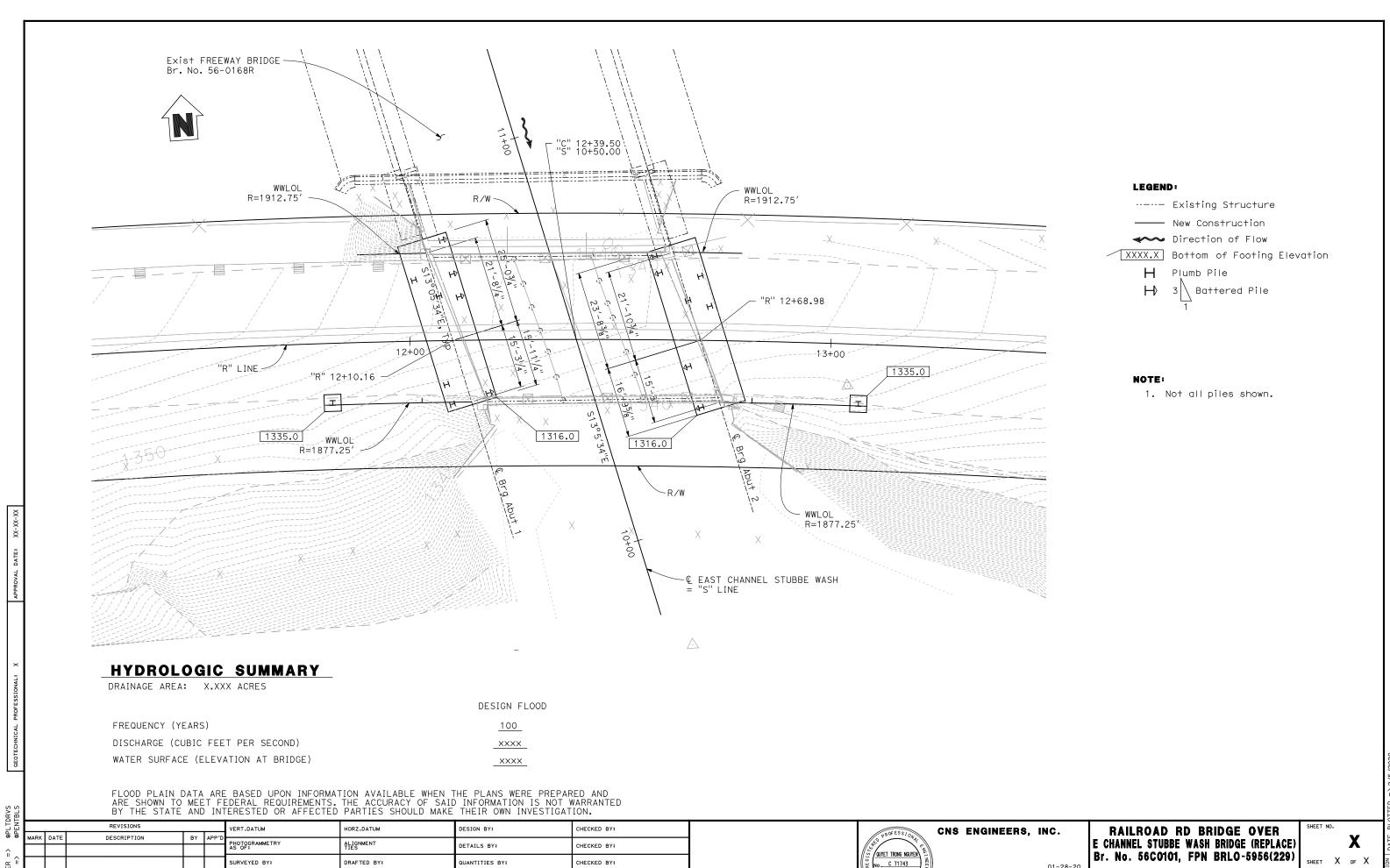
FOUNDATION PLAN

Y



WO 00000

COUNTY FILE No.



USERNAME => QuyetNguyen

DGN FILE => RR and Chuckwalla Avenue Bridge FPs.dgn

THE COUNTY OF RIVERSIDE OR ITS OFFICERS OR AGENTS SHALL NOT BE RESPONSIBLE FOR THE ACCURACY OR COMPLETENESS OF ELECTRONIC OR SCANNED COPIES OF THIS PLAN SHEFT.

CHECKED BY:

ORIGINAL SCALE IN INCHES

FIELD CHECKED BY:

WO 00000

PROJECT ENGINEER

11870 PIERCE ST., STE. 265 RIVERSIDE, CA 92505 PH: (951) 687-1009

FOUNDATION PLAN

COUNTY FILE No.

PROJECT SHEET NO.

Χ

TOTAL SHEETS

EXHIBIT "D": FEDERAL INSURANCE RATE MAP NO. 06065C0865G

NOTES TO USERS

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size. The community map repository should be consulted for possible updated or additional flood hazard information.

To obtain more detailed information in areas where Base Flood Elevations (BFEs) and/or floodways have been determined, users are encouraged to consult the Flood Profiles and Floodway Data and/or Summary of Stillwater Elevations tables contained within the Flood Insurance Study (FIS) report that accompanies this FIRM. Users should be aware that BFEs shown on the FIRM represent rounded whole-foot elevations. These BFEs are intended for flood insurance rating purposes only and should not be used as the sole source of flood elevation information. Accordingly, flood elevation data presented in the FIS report should be utilized in conjunction with the FIRM for purposes of construction and/or floodplain management.

Coastal Base Flood Elevations shown on this map apply only landward of 0.0' North American Vertical Datum of 1988 (NAVD 88). Users of this FIRM should be aware that coastal flood elevations are also provided in the Summary of Stillwater Elevations tables in the Flood Insurance Study report for this jurisdiction. Elevations shown in the Summary of Stillwater Elevations tables should be used for construction and/or floodplain management purposes when they are higher than the elevations shown on this FIRM.

Boundaries of the floodways were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the Flood Insurance Study report for this jurisdiction.

Certain areas not in Special Flood Hazard Areas may be protected by flood control structures. Refer to Section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures for this jurisdiction.

The projection used in the preparation of this map was Universal Transverse Mercator (UTM) zone 11. The horizontal datum was NAD 83, GRS80 spheroid. Differences in datum, spheroid, projection or UTM zones used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of this FIRM.

Flood elevations on this map are referenced to the North American Vertical Datum of 1988. These flood elevations must be compared to structure and ground elevations referenced to the same vertical datum. For information regarding conversion between the National Geodetic Vertical Datum of 1929 and the North American Vertical Datum of 1988, visit the National Geodetic Survey website at http://www.ngs.noaa.gov or contact the National Geodetic Survey at the following address:

NGS Information Services NOAA, N/NGS12 National Geodetic Survey SSMC-3, #9202 1315 East-West Highway Silver Spring, Maryland 20910-3282 (301) 713-3242

To obtain current elevation, description, and/or location information for bench marks shown on this map, please contact the Information Services Branch of the National Geodetic Survey at (301) 713-3242, or visit its website at http://www.ngs.noaa.gov.

Base map information shown on this FIRM was derived from U.S. Geological Survey Digital Orthophoto Quadrangles produced at a scale of 1:12,000 from photography dated 1994 or later.

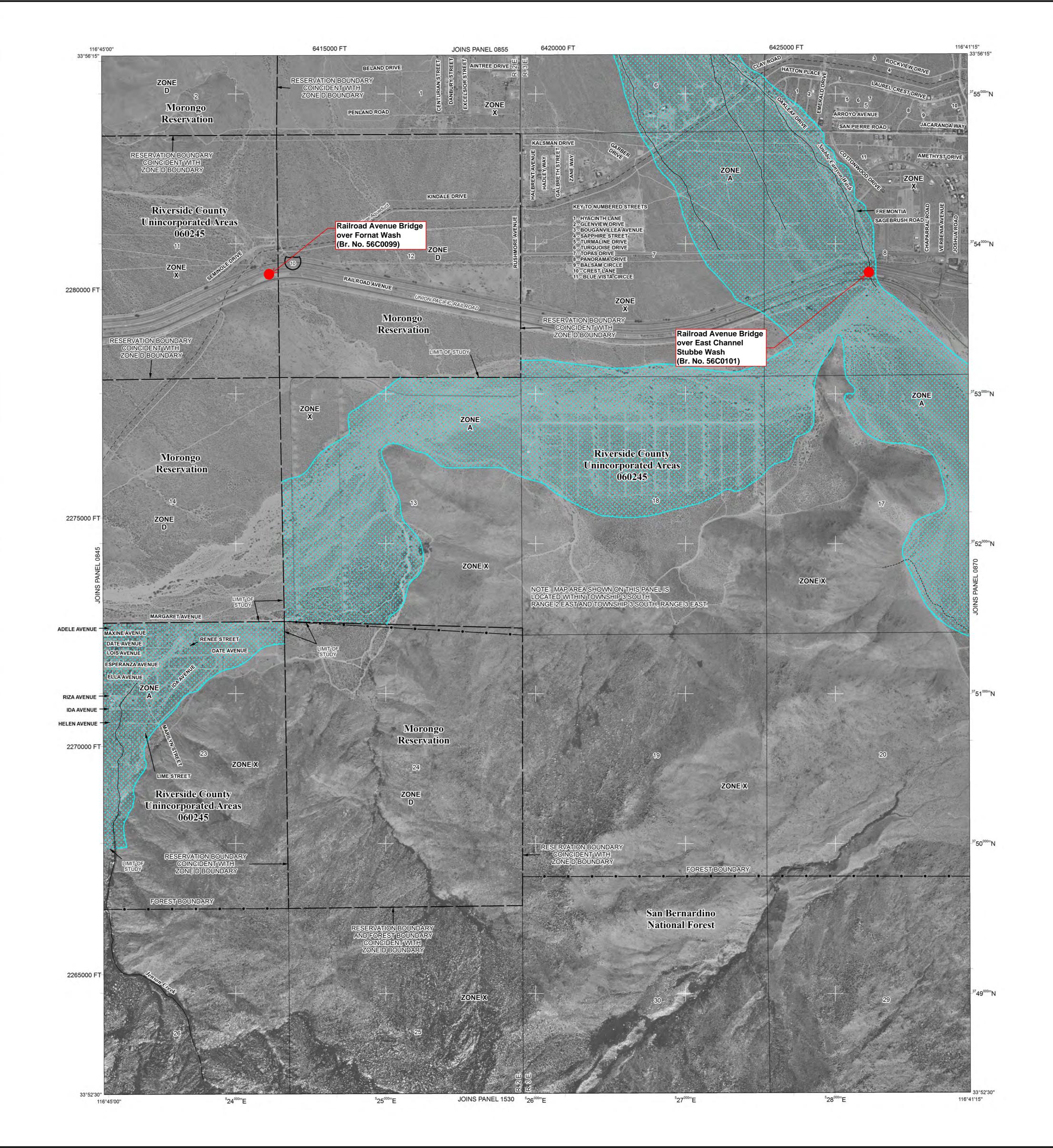
This map may reflect more detailed and up-to-date stream channel configurations than those shown on the previous FIRM for this jurisdiction. The floodplains and floodways that were transferred from the previous FIRM may have been adjusted to conform to these new stream channel configurations. As a result, the Flood Profiles and Floodway Data in the Flood Insurance Study Report (which contains authoritative hydraulic data) may reflect stream channel distances that differ from what is shown on this map.

Corporate limits shown on this map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after this map was published, map users should contact appropriate community officials to verify current corporate limit locations.

Please refer to the separately printed Map Index for an overview map of the county showing the layout of map panels; community map repository addresses; and a Listing of Communities table containing National Flood Insurance Program dates for each community as well as a listing of the panels on which each community is located.

Contact the FEMA Map Service Center at 1-800-358-9616 for information on available products associated with this FIRM. Available products may include previously issued Letters of Map Change, a Flood Insurance Study report, and/or digital versions of this map. The FEMA Map Service Center may also be reached by Fax at 1-800-358-9620 and its website at http://msc.fema.gov.

If you have questions about this map or questions concerning the National Flood Insurance Program in general, please call 1-877-FEMA MAP (1-877-336-2627) or visit the FEMA website at http://www.fema.gov.



LEGEND

SPECIAL FLOOD HAZARD AREAS SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD

The 1% annual flood (100-year flood), also known as the base flood, is the flood that has a 1% chance of being equaled or exceeded in any given year. The Special Flood Hazard Area is the area subject to flooding by the 1% annual chance flood. Areas of Special Flood Hazard include Zones A, AE, AH, AO, AR, A99, V, and VE. The Base Flood Elevation is the water-surface elevation of the 1% annual chance flood.

No Base Flood Elevations determined.

ZONE AE Base Flood Elevations determined.

ZONE AF Flood depths of 1 to 3 feet (usually areas of ponding); Base Flood Elevations determined.

ZONE AO Flood depths of 1 to 3 feet (usually sheet flow on sloping terrain); average depths determined. For areas of alluvial fan flooding, velocities also

Special Flood Hazard Area formerly protected from the 1% annual chance **ZONE AR** flood by a flood control system that was subsequently decertified. Zone AR indicates that the former flood control system is being restored to provide

protection from the 1% annual chance or greater flood. **ZONE A99** Area to be protected from 1% annual chance flood by a Federal flood protection system under construction; no Base Flood Elevations

ZONE V Coastal flood zone with velocity hazard (wave action); no Base Flood Elevations determined

Coastal flood zone with velocity hazard (wave action); Base Flood Elevations determined.

FLOODWAY AREAS IN ZONE AE

The floodway is the channel of a stream plus any adjacent floodplain areas that must be kept free of encroachment so that the 1% annual chance flood can be carried without substantial increases in flood heights.

OTHER FLOOD AREAS

ZONE X Areas of 0.2% annual chance flood; areas of 1% annual chance flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance flood.

OTHER AREAS

Areas determined to be outside the 0.2% annual chance floodplain. ZONE X Areas in which flood hazards are undetermined, but possible. ZONE D

COASTAL BARRIER RESOURCES SYSTEM (CBRS) AREAS

OTHERWISE PROTECTED AREAS (OPAs)

CBRS areas and OPAs are normally located within or adjacent to Special Flood Hazard Areas.

1% annual chance floodplain boundary 0.2% annual chance floodplain boundary

Floodway boundary

Zone D boundary

CBRS and OPA boundary

...... Boundary dividing Special Flood Hazard Area Zones and boundary dividing Special Flood Hazard Areas of different Base

Flood Elevations, flood depths or flood velocities.

Base Flood Elevation line and value; elevation in feet* Base Flood Elevation value where uniform within zone; elevation

* Referenced to the North American Vertical Datum of 1988

Cross section line

~~~ 513 ~~~

(EL 987)

(23)----(23) Transect line

Geographic coordinates referenced to the North American 87°07'45", 32°22'30" Datum of 1983 (NAD 83), Western Hemisphere

1000-meter Universal Transverse Mercator grid values, zone

600000 FT 5000-foot grid ticks: California State Plane coordinate

Bench mark (see explanation in Notes to Users section of this DX5510 FIRM panel)

●M1.5

MAP REPOSITORY

Refer to listing of Map Repositories on Map Index EFFECTIVE DATE OF COUNTYWIDE FLOOD INSURANCE RATE MAP

August 28, 2008 EFFECTIVE DATE(S) OF REVISION(S) TO THIS PANEL

For community map revision history prior to countywide mapping, refer to the Community Map History table located in the Flood Insurance Study report for this jurisdiction.

To determine if flood insurance is available in this community, contact your Insurance agent or call the National Flood Insurance Program at 1-800-638-6620.

PANEL 0865G

**FIRM** FLOOD INSURANCE RATE MAP

RIVERSIDE COUNTY, CALIFORNIA

AND INCORPORATED AREAS

PANEL 865 OF 3805

(SEE MAP INDEX FOR FIRM PANEL LAYOUT)

**CONTAINS**:

COMMUNITY RIVERSIDE COUNTY

(0)(0)(0)

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NAMICIOINAM

NUMBER PANEL SUFFIX 060245 0865 G

Notice to User: The Map Number shown below should be used when placing map orders; the Community Number shown above should be used on insurance applications for the

> MAP NUMBER 06065C0865G

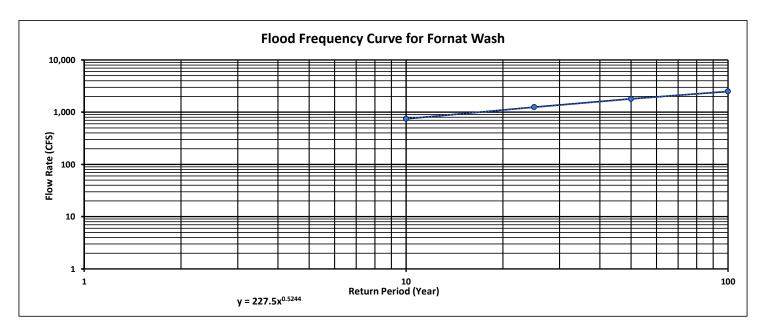
**EFFECTIVE DATE AUGUST 28, 2008** 

Federal Emergency Management Agency

**EXHIBIT "D"** 



**Exhibit E.1: Flood Frequency Curve for Fornat Wash** 



| FORNAT WASH FLOOD FREQUENCY CURVE (FFC) |       |                                                     |  |  |  |  |  |
|-----------------------------------------|-------|-----------------------------------------------------|--|--|--|--|--|
| Return Period Flow Rate                 |       |                                                     |  |  |  |  |  |
| (Year)                                  | (CFS) | Notes                                               |  |  |  |  |  |
| 100                                     | 2,500 | Calculated flow rate from Caltrans hydrology report |  |  |  |  |  |
| 50                                      | 1,800 | Calculated flow rate from Caltrans hydrology report |  |  |  |  |  |
| 25                                      | 1,250 | Calculated flow rate from Caltrans hydrology report |  |  |  |  |  |
| 10                                      | 750   | Calculated flow rate from Caltrans hydrology report |  |  |  |  |  |

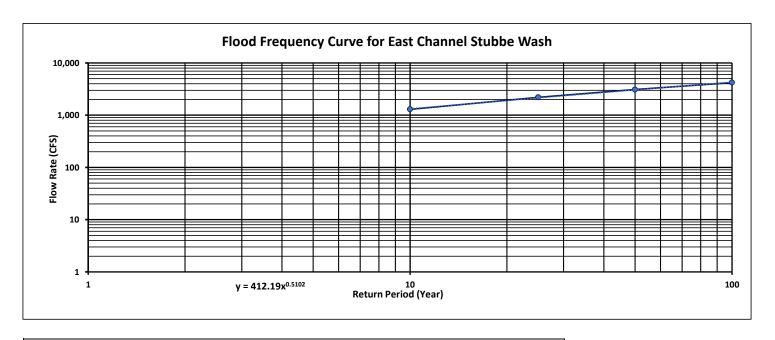
#### Overtopping Flow (CFS)

3,550

3,544 Resulting flow rate in CFS based on equation for the power equation

188 Inputted return period in years (iterative process) to equal flow capacity based on polynomial equation

**Exhibit E.2: Flood Frequency Curve for East Channel Stubbe Wash** 



| EAST CHANNEL STUBBE WASH FLOOD FREQUENCY CURVE (FFC) |       |                                                     |  |  |  |  |  |
|------------------------------------------------------|-------|-----------------------------------------------------|--|--|--|--|--|
| Return Period Flow Rate                              |       |                                                     |  |  |  |  |  |
| (Year)                                               | (CFS) | Notes                                               |  |  |  |  |  |
| 100                                                  | 4,200 | Calculated flow rate from Caltrans hydrology report |  |  |  |  |  |
| 50                                                   | 3,100 | Calculated flow rate from Caltrans hydrology report |  |  |  |  |  |
| 25                                                   | 2,200 | Calculated flow rate from Caltrans hydrology report |  |  |  |  |  |
| 10                                                   | 1,300 | Calculated flow rate from Caltrans hydrology report |  |  |  |  |  |

## Overtopping Flow (CFS) 12,800 12,796 Resulting flow rate in CFS based on equation for the power equation 840 Inputted return period in years (iterative process) to equal flow capacity based on polynomial equation





#### LOCATION HYDRAULIC STUDY FORM\*

| Dist. <u>8</u> | Co. | Riverside | Rte. | P.M |
|----------------|-----|-----------|------|-----|
|----------------|-----|-----------|------|-----|

Project No.: BRLO-5956(228) – Fornat Wash, BRLO-5956(229) – East Channel Stubbe Wash

Bridge No.: No. 56C0099 - Fornat Wash, No. 56C0101 - East Channel Stubbe Wash

Limits: The East Channel Stubbe Wash Bridge is located on Railroad Avenue approximately 0.3 miles west of the intersection of Railroad Avenue and Haugen-Lehmann Way. The Fornat Wash Bridge is located approximately 2.45 miles west of the East Channel Stubbe Wash Bridge along Railroad Avenue.

#### Floodplain Description:

Review of the Federal Emergency Management Agency (FEMA) Flood Insurance Rate Map (FIRM) Number 06065C0865G effective August 28, 2008 (Exhibit "D"), the following has been found:

- It is indicated that the Fornat Wash Bridge is within a designated Zone "X" (unshaded), which means the area is outside the 0.2% annual chance floodplain. Therefore, the Fornat Wash Bridge does not encroach into any defined floodplains or floodways as shown on Exhibit "D".
- It is indicated that the East Channel Stubbe Wash Bridge is within a designated Special Flood Hazard Zone "A" known as the Stubbe Canyon Wash, which means that the area is subject to the 1% annual chance floodplain, with no known base flood elevations. Therefore, the East Stubbe Channel Wash Bridge encroaches into the 1% annual chance flood hazard zone as shown on Exhibit "D", and thus subject to inundation by the 100-year flood event. Construction of the bridge will not have any impact to the existing floodplain.
- Caltrans has prepared a hydrology study for Fornat Wash and East Channel Stubbe Wash showing the following flow rates for the 100-year and 50-year flood events (Ref. 1):
  - o Fornat Wash
    - $Q_{100} = 2,500 \text{ CFS}$
    - $\bullet$  O<sub>50</sub> = 1.800 CFS
  - o <u>East Channel Stubbe Wash</u>
    - $Q_{100} = 4,200 \text{ CFS}$
    - $O_{50} = 3,100 \text{ CFS}$

The flow rates were determined using the "Troxell method", an outdated hydrologic calculation method. A floodplain analysis using the HEC-RAS model was performed for Fornat Wash and East Channel Stubbe Wash using Caltrans' flow rates based on the proposed bridge footprint at Railroad Avenue. The results indicate that the 100-year and 50-year flow rates are contained within the cross section of the channel and the proposed bridge section with the following bridge freeboard:

#### o Fornat Wash

For  $Q_{100} = 2,500$  CFS, the average freeboard at the upstream side of the bride is approximately 0.13 feet with a maximum freeboard of 0.26 feet at the northwest bridge abutment while at the downstream side of the bridge, it is approximately

0.72 feet with a maximum freeboard of 1.28 feet at the southwest bridge abutment.

For Q<sub>50</sub> = 1,800 CFS, the average freeboard at the upstream side of the bride is approximately 0.82 feet with a maximum freeboard of 1.31 feet at the northwest bridge abutment while at the downstream side of the bridge, it is approximately 1.60 feet with a maximum freeboard of 2.16 feet at the southwest bridge abutment.

#### o <u>East Channel Stubbe Wash</u>

- For Q<sub>100</sub> = 4,200 CFS, the average freeboard at the upstream side of the bride is approximately 5.54 feet with a maximum freeboard of 5.93 feet at the northeast bridge abutment while at the downstream side of the bridge, it is approximately 7.74 feet with a maximum freeboard of 8.18 feet at the southeast bridge abutment.
- For Q<sub>50</sub> = 3,100 CFS, the average freeboard at the upstream side of the bride is approximately 6.91 feet with a maximum freeboard of 7.33 feet at the northeast bridge abutment while at the downstream side of the bridge, it is approximately 8.91 feet with a maximum freeboard of 9.34 feet at the southeast bridge abutment.

The overtopping flow rates at the two bridges were determined to be approximately 3,550 CFS for Fornat Wash and 12,800 CFS for East Channel Stubbe Wash. These flows are equivalent to approximately 188-year and 840-year flood events, respectively.

**1. Description of Proposal** (include any physical barriers i.e. concrete barriers, soundwalls, etc. and design elements to minimize floodplain impacts):

Railroad Avenue is an approximately 5-mile stretch of road that runs parallel to Interstate 10 (I-10) and the Union Pacific Railroad (UPRR). It connects the Haugen-Lehmann Way and I-10 at the east end and Main Street and I-10 at the west end. It mostly serves the sparsely populated Cabazon community. The average daily traffic (ADT) volume is approximately 339 vehicles. Periodically, the road carries detoured traffic from the heavily traveled I-10 when the freeway is temporarily closed for construction or emergency incidents. The road also serves as an access route for UPRR and utility maintenance crews. Therefore, it is important to maintain this frontage road in sound condition at all times.

The existing timber bridges carry two lanes (one lane in each direction) of traffic over Fornat and East Channel Stubbe Washes. The timber bridges are approximately 59 feet long and are 32 feet wide from curb-to-curb. The County proposes replacing the existing two 2-lane timber bridges along Railroad Avenue with new 2-lane modern bridges with a curb-to-curb roadway width of 32 feet at the same locations.

The bridges are listed in the federal Eligible Bridge List (EBL), as "Structurally Deficient (SD)" with a low Sufficiency Rating (SR) between 59.1 and 62.9. A sufficiency rating is essentially an overall rating of a bridge's fitness for the duty that it performs. The rating is based on a bridge's structural evaluation, functional/geometric obsolescence, and its essentiality to the public. A low sufficiency rating may be due to structural defects, narrow lanes, low vertical clearance, or any of many possible issues. A bridge is healthy when its SR is more than 80.0. Bridges with SR equal to or less than 80.0 and more than 50.0

require rehabilitation or widening. When the SR falls less than 50.0, bridge replacement shall be considered for public safety. Although the Railroad Avenue bridges carry a status flag of SD with SR ratings between 50 and 80 (qualifying for major rehabilitation), it was determined that the bridges are well beyond their 50-year service life and it would be more cost-efficient to replace the bridges. Additionally, a scour Plan of Action (POA) was performed on the bridges by the County in 2013. The POA recommended total replacement of the bridges as the most cost-effective option due to the extent of the scour, structural instability and deterioration of various timber bridge elements.

The proposed project would replace the existing 2-lane timber bridges with new 2-lane modern bridges. The proposed road width would consist of two 12-foot-wide travel lanes, one lane in each direction, and a 4-foot-wide shoulder on each side. Modern traffic barriers/railings meeting current CALTRANS safety design standards would be constructed. The proposed bridges would be approximately 60 feet long depending on the channel hydraulic capacity and water surface freeboard requirements. Potentially the elevation of Fornat Wash Bridge may increase, but by no more than two feet to meet freeboard requirements. The East Channel Stubbe Wash Bridge elevation would remain the same. Additionally, approach roadway improvements would be provided and channel improvements would be administered to avoid future scour problems. It is envisioned that the channel bottom will remain earthen.

Existing underground utilities along the north side of Railroad Avenue and suspended utilities (a 4-inch gas line and a telephone line) along the north side of the East Channel Stubbe Wash bridge would be affected by construction and may require relocation.

All construction activities would be conducted within the existing roadway right of way with construction staging and material laydown areas on the roadway itself. Railroad Avenue between the two bridges to be replaced would be closed for continuous traffic during construction. The construction duration will be further determined during the project development. It is envisioned that the two bridges will be constructed one at a time to allow access to UPRR facilities and adjacent utilities from the Haugen-Lehmann Way/I-10 Interchange or the Main Street/I-10 Interchange. A Traffic Management Plan (TMP) would be prepared to address closure of the road and access to local utilities and properties.

The proposed construction would require a temporary construction easement (TCE) from UPRR for access to the channel bottom. However, construction activities are expected to stay at least 50 feet from the live rail tracks to eliminate any effects on railroad operations. The Railroad Avenue bridges abut adjacent State Bridges (Br. No. 56-166 and Br. No. 56-168) that carry I-10 traffic over the same washes. Structural modifications to the State Bridges are not anticipated; however, this will be evaluated during design. An encroachment permit from Caltrans District 8 would be obtained prior to construction.

| 2. ADT:      | Current339                  | Projected    | 504  |
|--------------|-----------------------------|--------------|------|
| 3. Hydraulio | e Data:                     | ·            |      |
| 3a. Forna    | t Wash                      |              |      |
|              | Base Flood Q100= 2,500 CF   | S            |      |
|              | WSE100: Downstream Encroach | hment = 1551 | 1.86 |
|              | Upstream Encroachment = 155 | 53.93        |      |
| 3b. East (   | Channel Stubbe Wash         |              |      |
|              | Base Flood Q100= 4,200 CF   | S            |      |
|              | WSE100: Downstream Encroach | hment = 1339 | 9.71 |
|              | Upstream Encroachment = 134 | 42.79        | •    |

|                            | lood of record, if greater than N/A CFS WSE=                                                                        | Q100:                | N/A              | _                               |                              |                      |             |        |                     |     |
|----------------------------|---------------------------------------------------------------------------------------------------------------------|----------------------|------------------|---------------------------------|------------------------------|----------------------|-------------|--------|---------------------|-----|
|                            | topping flood Q for Fornat Wa<br>topping flood Q for East Chan                                                      |                      |                  |                                 | ,800                         |                      | WSE=        |        | <u>4</u><br>1351.17 |     |
| Are N                      | NFIP maps and studies availab                                                                                       | le?                  | YES_             | X                               |                              | I No. 06<br>xhibit " |             | 365G   |                     |     |
| 4. Is t                    | the highway location alternativ                                                                                     | ve withi             | _                | ılatory                         | floodwa                      | y?                   |             |        |                     |     |
| 5. Att                     | tach map with flood limits ou<br>plain.                                                                             | tlined s             | howing           | ; all bui                       | ldings o                     | or other             | improv      | ements | within the ba       | ıse |
| Poten                      | tial Q100 backwater damages                                                                                         | :                    |                  |                                 |                              |                      |             |        |                     |     |
| A.<br>B.<br>C.<br>D.<br>E. | Residences? Other Bldgs? Crops? Natural and beneficial Floodplain values?                                           | NO<br>NO<br>NO<br>NO | X<br>X<br>X<br>X | YES<br>YES<br>YES<br>YES<br>YES |                              | _<br>_<br>_<br>_     |             |        |                     |     |
| 6. Ty <sub>1</sub>         | pe of Traffic:                                                                                                      |                      |                  |                                 |                              |                      |             |        |                     |     |
| B. En<br>C. Pra            | nergency supply or evacuation<br>nergency vehicle access?<br>acticable detour available?<br>hool bus or mail route? | n route?             | NO<br>NO<br>NO   | X                               | YES_<br>YES_<br>YES_<br>YES_ | X<br>X               | -<br>-<br>- |        |                     |     |
| 7. Est                     | imated duration of traffic inte                                                                                     | rruption             | for 10           | 0-year e                        | event ho                     | urs:                 | N/A         | _      |                     |     |
| 8. Est                     | timated value of Q100 flood d                                                                                       | amages               | (if any)         | ) – mod                         | erate ris                    | sk level.            |             |        |                     |     |
| A.<br>B                    | Roadway         \$         N/A           Property         \$         N/A           Total         \$         N/A     | _<br>_<br>_          |                  |                                 |                              |                      |             |        |                     |     |
| 9.                         | Assessment of Level of Risk                                                                                         | Mode                 | X<br>rate        | _                               |                              |                      |             |        |                     |     |

For High Risk projects, during design phase, additional Design Study Risk Analysis May be necessary to determine design alternative.

| Signature – Consulting Hydraulics Engli                                 | Date: 02/24/2020                                                    |
|-------------------------------------------------------------------------|---------------------------------------------------------------------|
| (Item numbers 3,4,5,7,9)                                                | Ceazar Aguilar, P.E.,                                               |
|                                                                         | Aguilar Consulting, Inc                                             |
|                                                                         |                                                                     |
| Is there any longitudinal encroachmen Floodplain development?           | t, significant encroachment, or any support of incompatibl NO X YES |
| If yes, provide evaluation and discussion 650.113                       | on of practicability of alternatives in accordance with 23 CFI      |
| Information developed to comply with the retained in the project files. | he Federal requirement for the Location Hydraulic Study shall       |
| Signature – County Project Engineer<br>(Item numbers 1,2,6,8)           | Umer Ahmed, Sr. Civil Engineer, RCTD                                |

<sup>\*</sup> Same as Figure 804.7A Technical Information for Location Hydraulic Study located in Chapter 804 of the Highway Design Manual



#### SUMMARY FLOODPLAIN ENCROACHMENT REPORT\*

| Dist. 8 Co. Riverside Rte. P.M.                                                                                                                                                                                     | 74-1.1.                        |
|---------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|--------------------------------|
| Project No.: <u>BRLO-5956(228) – Fornat Wash</u> , <u>BRLO-5956(229) – East Channel Swaph</u>                                                                                                                       | <u>stubbe</u>                  |
| Wash Bridge No.: No. 56C0099 – Fornat Wash, No. 56C0101 – East Channel Stubbe Wash                                                                                                                                  | a <b>h</b>                     |
| Limits: The East Channel Stubbe Wash Bridge is located on Railroad A                                                                                                                                                |                                |
| approximately 0.3 miles west of the intersection of Railroad Avenue and Haugen-Lel                                                                                                                                  |                                |
| Way. The Fornat Wash Bridge is located approximately 2.45 miles west of the                                                                                                                                         |                                |
| Channel Stubbe Wash Bridge along Railroad Avenue.                                                                                                                                                                   |                                |
|                                                                                                                                                                                                                     |                                |
| Floodplain Description:                                                                                                                                                                                             |                                |
| Review of the Federal Emergency Management Agency (FEMA) Flood Insurance                                                                                                                                            | e Rate                         |
| Map (FIRM) Number 06065C0865G effective August 28, 2008 (Exhibit "D'                                                                                                                                                |                                |
| following has been found:                                                                                                                                                                                           |                                |
|                                                                                                                                                                                                                     |                                |
| • It is indicated that the Fornat Wash Bridge is within a designated Zone "X"                                                                                                                                       |                                |
| (unshaded), which means the area is outside the 0.2% annual chance floodplain.                                                                                                                                      |                                |
| Therefore, the Fornat Wash Bridge does not encroach into any defined floodpla                                                                                                                                       | ins or                         |
| floodways as shown on Exhibit "D".                                                                                                                                                                                  |                                |
| Let is indicated that the Foot Study Week Duides is within a designated Supplied F                                                                                                                                  | 71 a a d                       |
| • It is indicated that the East Stubbe Wash Bridge is within a designated Special F                                                                                                                                 |                                |
| Hazard Zone "A" known as the Stubbe Canyon Wash, which means that the are                                                                                                                                           |                                |
| subject to the 1% annual chance floodplain, with no known base flood elevation. Therefore, the East Stubbe Channel Wash Bridge encroaches into the 1% annual chance floodplain, with no known base flood elevation. |                                |
| chance flood hazard zone as shown on Exhibit "D", and thus subject to inundati                                                                                                                                      | _                              |
| the 100-year flood event. Construction of the bridge will not have any impact to                                                                                                                                    |                                |
| existing floodplain.                                                                                                                                                                                                | tiic                           |
| <u></u>                                                                                                                                                                                                             |                                |
|                                                                                                                                                                                                                     | No Yes                         |
| 1. Is the proposed action a longitudinal encroachment of the base floodplain?                                                                                                                                       | <u>X</u>                       |
|                                                                                                                                                                                                                     |                                |
| 2. Are the risks associated with the implementation of the proposed action                                                                                                                                          | 1 <u>x</u>                     |
| significant?                                                                                                                                                                                                        |                                |
| 3. Will the proposed action support probable incompatible floodplain                                                                                                                                                | 1 <u>x</u>                     |
| development?                                                                                                                                                                                                        |                                |
| 4. Are there any significant impacts on natural and beneficial floodplain values?  5. Positive construction proceedures are provined to minimize impacts on the                                                     |                                |
| 5. Routine construction procedures are required to minimize impacts on the                                                                                                                                          |                                |
| floodplain. Are there any special mitigation measures necessary to minimize impacts or restore and preserve natural and beneficial floodplain values? If yes                                                        |                                |
| explain.                                                                                                                                                                                                            | ,                              |
| 6. Does the proposed action constitute a significant floodplain encroachment as                                                                                                                                     | s <u>x</u>                     |
| defined in 23 CFR, Section 650.105(q).                                                                                                                                                                              | <u>A</u>                       |
| 7. Are Location Hydraulic Studies that document the above answers on file? I                                                                                                                                        | f <u>x</u>                     |
| not explain.                                                                                                                                                                                                        | _ <del></del> _ <del>^</del> _ |

#### PREPARED BY:

Signature – Consulting Hydraulics Engineer

Date: 02/24/2020

Ceazar Aguilar, P.E., Aguilar Consulting, Inc

Signature – County Project Engineer

Date 03/03/2020

Umer Ahmed,

Sr. Civil Engineer, RCTD

<sup>\*</sup> Same as Figure 804.7B Floodplain Evaluation Report Summary located in Chapter 804 of the Highway Design Manual